

July 28, 2025

Dr. Michael Klemens, Chairman
Planning and Zoning Commission
Town of Salisbury
27 Main Street
P.O. Box 548
Salisbury, CT 06068

SLR Project No.: 141.22100.00001

**RE: Responses to Town Engineering Review Comments
PZC Application #2025-0287
Wake Robin Inn Redevelopment
104 & 106 Sharon Road
Salisbury, Connecticut**

Dear Mr. Chairman and Commissioners,

SLR International Corporation (SLR) is in receipt of correspondence with review comments from Thomas D. Grimaldi, Principal Engineer and Robert R. Hiltbrand, Principal of R.R. Hiltbrand Engineers & Surveyors, L.L.C. dated June 24, 2025, regarding the above-referenced application. We offer the following responses to the comments contained therein, along with the enclosed revised site plans and drainage report dated July 28, 2025.

Engineering Review Comments:

- C1. We received the lab permeability testing results. Per the CT 2024 Stormwater Quality Manual, in-situ permeability testing is required.
- R1. In-situ double-ring infiltrometer testing was performed by SLR on July 14, 2025 in DET 210, DET 220 and WQB 140. Refer to revised site plans for testing locations and data.**
- C2. Times of Concentrations for EXWS-30 and PRWS-12 exceed maximum recommended of 100 ft for sheet flow. (See Part 630 of the USDA NRCS National Engineering Handbook, Chapter 15, Time of Concentration).
- R2. The time of concentration computation has been revised for EXWS-30. PRWS-12 is no longer included in the drainage analysis since it is no longer applicable to the project.**
- C3. Provide time paths on drainage maps.
- R3. The time of concentration paths on the EXWS and PRWS watershed maps have been changed to be shown in a red color for clarity.**
- C4. Provide roof leader outlet piping from proposed Spa Building into WQB 140.

- R4.** Per the CT 2024 Stormwater Quality Manual, roof runoff is generally considered to be clean and does not require treatment prior to discharge. Further, the roof and deck will be disconnected from a stormwater conveyance system and can be removed from the equation for WQV (*CTDEEP SWM pg. 47*) - therefore it is not necessary for these areas to be directed to WQB 140. The runoff from both the roof and deck will be directed underneath the building to a recessed 4" layer of river stone over filter fabric (replacing the existing organic layer in the areas below the building and deck). Downspouts will discharge to precast dissipator pads and runoff will generally disperse within the river stone layer and drain into the underlying soil with excess flow traveling overland along existing drainage pathways.
- C5. Provide weir elevations on detail and site plan for WQB 140.
- R5.** Elevations have been provided on Sheet UT and on the emergency riprap spillway detail for each basin.
- C6. Elevations of wall spillway shall be provided on both the grading and utility plans to avoid any confusion.
- R6.** The elevation of the wall spillway has been added to both Sheet GR and Sheet UT.
- C7. Provide outlet protection for 2nd pipe entering D-Basin 210. Label pipe size, slope, and outlet flowline.
- R7.** Outlet protection has been added for FES 76. Callouts for YD 72, FES 76, and the connecting pipe have been added to Sheet UT.
- C8. Label yard drain southeast of bathrooms/storage building.
- R8.** A label for YD 72 has been added to Sheet UT.
- C9. Provide pipe size and slope for pipe connecting CLCB 14 to MH 13.
- R9.** A callout for the size and slope of this pipe is provided on Sheet UT.
- C10. How will the design engineer address the WQV for impervious surfaces from Wells Hill Road to the proposed bridge?
- R10.** The grading has been revised downgradient of the proposed impervious surfaces between Wells Hill Road and the proposed bridge to direct runoff over grass lawn areas which will serve as vegetated filter strips before the runoff reaches the adjacent wetland, consistent with existing runoff patterns in this area.
- C11. Provide inlet/outlet protection for FES 31A & 32.
- R11.** Inlet and outlet protection for FES 31A & 32 have been added.
- C12. How will the design engineer address the WQV for the entrance loop located off of Sharon Road?



- R12. A hydrodynamic separator has been added to the storm drainage system prior to discharging from the property at this location.**
- C13. Provide ECB on all slopes of 5% or greater.
- R13. Permanent erosion control blankets are shown on Sheet SE-1 for slopes 2' horizontal to 1' vertical (50%) or greater. A note has been added to the Soil Erosion and Sediment Control Narrative on Sheet SE-1 as follows: "Temporary erosion control blankets (ECB) shall be installed and maintained in all disturbed site areas with slopes greater than 5% (1' vertical in 20' horizontal) that are to be re-vegetated until the surfaces are permanently stabilized."**
- C14. Provide a construction staging area on the Site Plan-Removals sheet.
- R14. A construction staging area has been added to the Site Plan – Removals sheet.**
- C15. How will the design engineer address potential erosion and/or sediment migration on the steep section of the proposed access drive located off of Sharon Road? Consider water bars and a temporary sediment trap(s).
- R15. Water bars and callouts have been added to Sheet SE-1 and a water bar detail has been added to Sheet SE-2.**
- C16. Provide a detailed operation and maintenance plan (OMP) for the entire storm drainage system.
- R16. An operation and maintenance plan has been provided for the storm drainage system as part of the drainage report.**
- C17. We have reviewed the Erosion and Sedimentation Control Measures Bond estimate and have set the bond at \$122,214.84. Said bond shall be a cash bond payable to the "Town of Salisbury". No permits shall be issued until the bond has been posted.
- R17. Comment acknowledged.**

Please feel free to contact me at (203) 271-1773 if you need additional information regarding the above responses.

Regards,

SLR International Corporation



Todd Ritchie, PE, BCEE, CFM, REHS/RS
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Enclosures

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